



# Rockmass Characteristics and its Influence on TBM Penetration Rate in Archeans and Meta-sedimentary Rocks of India

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### Abstract

TBM penetration rate prediction models were developed for the hard Archean granites, granite gneisses and Meta-sedimentary rock groups comprising of quartzites, quartz arenites and sandstone of peninsular India.Construction stage engineering geological mapping comprising of face logging resulted in rockmass characterisation of Archeans and Meta-sedimentary rock groups. Statistical correlation of TBM penetration rate (PR) with Rock quality designation (RQD), unconfined compressive strength (UCS), volumetric joint count (Jv) and rockmass class was carried out. The prediction model provides good correlation only in the Meta sedimentary rock group for RQD, UCS and rockmass class and good correlation only for RQD and rockmass class in the Archean group but poor correlation with UCS. There is an exponential increase of penetration rate with Jv in both the rock groups.

Keywords: TBM Penetration Rate, Rockmass Characteristics, Geological Mapping, Rockmass Classification, Rock Tunnel

### Introduction

One of the key challenges of excavation by Tunnel Boring Machine (TBM) in hard rock is the prediction of reliable penetration rates, which is often a decisive factor in the overall cost of the project. Though various borability models were developed by researchers (Rostami and Ozdemir, 1993b-c; Bruland, 1998; Barton, 1999; Bieniawski *et al.*, 2007; Yagiz, 2008; Gong and Zhao, 2009), each model has its own specific limitations on geology and is not universally applicable.

In rock tunnels, important factors governing TBM excavability are rockmass characteristics, machine variables and efficiency of operator. Two of the currently most recognized TBM performance prediction and prognosis models in use around the world are the Colorado School of Mines (CSM) (Rostami and Ozdemir, 1993; Rostami, 1997) and Norwegian University of Science and Technology (NTH) models (Blindheim, 1979; Bruland, 1998). CSM model for TBM performance prediction was based on the force equilibrium method and involves measurement and evaluation of cutting forces on an individual cutter (Rostami and Ozdemir, 1993b-c). The estimate of cutting forces is provided by full scale cutting test. This model uses rock property values such as UCS and Tensile strength and combined with cutter geometry and cutter information, gives an estimate of the cutting forces. This model does not take the rockmass properties into account and the effect of grain size, hence applicable only for massive rocks. NTH

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developed an empirical model by considering intact rock, rockmass properties and machine parameters (Lislerlud, 1988; Bruland, 1998) and are based on the historical field performance of TBMs in certain rock types. The models are a set of empirical graphs and equations between rock properties, ground conditions, machine parameters and penetration rate, as these methods rely on past data, their forecasting abilities are limited. The main advantage of this method is that it incorporates the ground conditions and the excavation system as a whole and the prediction estimates are accepted by TBM operators. Barton (1999) gave an equation for estimating the rate of penetration expressed by  $Q_{\text{TBM}}$  which is a modification of existing Q system (Barton et al., 1974) of rockmass classification for drill and blast tunnels. Many workers have used rockmass rating (RMR) parameter for estimating TBM performance (Bieniawski et al., 2007; Hamidi et al., 2010). Yagiz (2008) added brittleness of intact rock and fracture properties of rockmass as indices into the model. Gong and Zhao (2009) developed a rockmass characteristics model for prediction of TBM penetration rate in granites of Singapore. Performance evaluation of TBM in Deccan Trap rocks was also made along with evaluation of geotechnical properties (Jain, 2014). The RQD analysis has also been associated with the several limitations (Pells et al., 2017). Geotechnical parameters affecting the penetration rate of TBM using neural networks was evaluated based on a case study of Newsoud project in Iran (Hossein et al., 2020). Comparison of various existing empirically based models for estimation of TBM utilization was made using discrete event simulation (Khetwal et al., 2021). Prediction of TBM performance was made using empirical and statistical methods of linear multiple regression and

nonlinear multiple regression in fresh and weathered granites (Armaghani *et al.*, 2021). Specific rockmass borability index employing regression analysis was developed with incorporation of machine parameters in rockmass characteristics model (Qiuming *et al.*, 2022).

In India, the Government of Andhra Pradesh embarked on AMRP (Alimineti Madhava Reddy Project) with Tunnel1 of length 43.93km as one of the components. After bifurcation of state, the project is being implemented by the Government of Telengana. The AMRP project is envisaged to irrigate 1,09, 250 hectares (2.70 lakh acres) in 15 Mandals of Nalgonda District, besides providing drinking water to 516 villages having excessive fluoride in groundwater. Two Robbins TBMs were deployed for boring from Inlet and Outlet ends of Tunnel1with a finished diameter of 9.2M and circular cross section. It is a water transfer tunnel and once completed will be the longest tunnel without intermediate access in the globe (robbinstbm.com). M/s Jaiprakash Associated Ltd is the EPC contractor for this project. This tunnel will transfer water from the left bank of the Krishna River falling in Mahbubnagar District to the Outlet near Mannevaripalli Village, Mahbubnagar District as shown in the layout plan (Fig.1) from where the water will be transferred to the plains of Nalgonda District through a system of canals and Tunnel 2 as part of the AMRP scheme. This long tunnel project provides opportunities for study of rockmass characteristics, geotechnical properties and predictive models of TBM borability in hardrocks such as, archean granites and gneisses and metasedimentary rocks like sandstone, quartzite and quartz arenites.

### Scope of the Study

The present study seeks to predict the TBM penetration in two hardrock groups namely, Archean Granites and Gneisses and Meta-sedimentary group comprising of Sandstone, Quartzite and Quartz arenites through the study of Tunnel1 bored in Mahbubnagar District, Telengana state, India.

Sedimentary rocks of weak strength (shale and siltstone) are excluded in the study. The variation of machine parameters like thrust, torque, condition of the machine as well as efficiency of TBM operator is not considered in this work. As the TBMs are double shielded, opportunities to observe the face are limited. Accordingly, construction stage geological mapping and classification of rockmass was carried out (Wilfing, 2016).

### Geology of the Study Area and Tunnel T1

The geological setting comprises of meta-sedimentary rocks namely, Srisailam Quartzites which are interbedded with shale and siltstone and form a part of the Cuddapah Supergroup resting unconformably above the Archean granites and gneisses. This constitutes the tunneling media for Tunnel 1 with Meta-sedimentary group from the Inlet end and Archean granites and gneisses from the Outlet end.

The Srisailam Quartzites constitute the uppermost stage of the Krishna Series in the Cuddapah Super Group and rest unconformably on the Archaean Basement (King, 1872; Sen and Narsimha Rao, 1967). The Cuddapah Basin is one of the Meso-Neoproterozoic sedimentary basins in the east-central part of the Dharwar Craton, crescent in shape, easterly concave, and trends N-S. The arcuate north, south, and western boundary of the Cuddapah Basin marks the profound unconformity (Eparchean Unconformity



Fig. 1. Location Map of Study Area, Telangana State, India

in early literature) on the basement granites. The arcuate eastern margin is marked by a prominent boundary thrust which is parallel to the Nellore schist belt. A giant basic dyke swarm underlies the basin in the west from Chittoor in the south to Mahbub Nagar in the north (Ramakrishnan and Vaidyanathan, 2008).

Rajurkar and Ramalingaswami (1975) divided the Cuddapah succession into three groups, namely, the lower Cuddapah Group, Nallamalai Group and Kurnool Group. The Srisailam Quartzites are included under the Cumbum slate subgroup of the Nallamalai Group. Meijerink *et al.* (1984) also proposed a three-fold division of the succession and included the Srisailam Formation under the Bairenkonda Subgroup. The Nallamalai Fold Belt (NFB) forms a tectonic boundary with the other rock groups and was excluded from the lithostratigraphic succession (Patranabis-Deb *et al.*, 2012). The NFB is separated from the Cuddapah Basin by the easterly dipping Rudravaram thrust (Das and Chakraborty, 2019). The lithostratigraphic divisions of the Cuddapah System following Nagaraja Rao *et al.* (1987) are adopted in this work (Table 1).

Mandapalli Raju (1989, 2008, 2009) investigated the geotechnical conditions along the tunnel alignment in the SLBC tunnel scheme in the AMRP. Raju (2009) made a geotechnical evaluation of the design parameters for use in the TBMs of the SLBC tunnel with the help of satellite imageries. The geotechnical parameters was mapped and monitored in the construction stage in Tunnel 2 (Ramkrishna and Rao, 2023).

### Methods

The study area falls in Survey of India, topographic sheet numbers 56L/14/SE, 56L/15/NE, 56L/15/SE, 56L/16/NE, 56L/16/SE of Mahbub Nagar district in Telangana state. Tunnel 1 has its Inlet end located at  $78^{\circ}53'26''$  E,  $16^{\circ}05'44''$  N on the left bank

Group	Formation	Thickness(m)	Lithology				
Kurnool Group	Nandyal Shale Koilkuntla Limestone Paniam Quartzite Owk Shale Narji Limestone Banganapalle Quartzite	$50\pm100 \\ 15\pm50 \\ 10\pm35 \\ 10\pm15 \\ 100\pm200 \\ 10\pm50$	Shale Limestone Quartzite Shale (Ocherous) Limestone Conglomerate, Quartzite				
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		~ Unconformity ~~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~				
Srisailam Quartzites		300	Quartzites and Shale				
~~~~~~~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	~ Unconformity ~~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~				
Nallamalai Group	Cumbum: Phyllite, Slate, quartzite, Dolomite Cumbum (Pullampet) Formation Bairenkonda (Nagari) Quartzite	2000 1500± 4000	Nagari:conglomerate, quartzites, and shales with intrusives				
	А	ngular unconformity					
Chitravati Group	Gandikota Quartzite Tadpatri Formation Pulivendla Quartzite	300 4600 1±75	Shale, ash fall tuffs, quartzite, dolomite with intrusives Conglomerate and quartzite				
		Disconformity					
Papaghni Group	Vempalli Formation	1900	Stromatolitic dolomite, dolomitic mudstone, chert, breccia, and quartie with basic flows and intrusives				
	Gulcheru quartzites	28±210	Conglomerate, arkose, quartzite and shale				
		Nonconformity					
	Arcl	nean Gneissic Complex					

Table1: Stratigraphy of Cuddapah Supergroup (Modified after Nagaraja Rao et al., 1987)

of the Krishna River near Domalapenta Village of Amrabad Mandal upstream of the Srisailam Left Bank Powerhouse while the Outlet end is at 78°56'56" E,16°32'23" N near Mannevaripalli Village, Achampeta Mandal (Fig.1).

As the TBMs are double shielded, opportunities to observe the face are available only during maintenance workand stoppage of machine *viz.* retraction of cutter head, change of disc cutters, seal change, change of the main bearing of TBM. Construction stage geological mapping and collection of rockmass parameters was done as per IS 11315 whenever the opportunity was available. This comprised of rockmass parameters and rockmass quality Q (IS13365, Part2:1992), collected over 126 tunnel section data sets from Outlet end and 66 data sets from Inlet end (Table 2-3). In addition, core and lump samples were recovered from select reaches and were utilized for determining the engineering properties and physical properties. Core samples were recovered using HILTI DD130 core drilling machine.

Physical properties which were determined are bulk density, dry density, porosity, and water content. Engineering Properties determined are unconfined compressive strength (UCS), Tensile strength, and Point Load Strength Index. The results were correlated with penetration rate and statistical models were developed to predict the penetration rate.

### **Results and Discussion**

### Variation of TBM Penetration Rate with RQD

Rock Quality Designation (RQD) was determined by indirect methods based on degree of jointing (IS11315, Part 11). The RQD values determined for Tunnel 1 for the Meta-sedimentary rock group (Table 2) and the Archean rocks (Table 3). Figure 2 shows the variation of TBM penetration rate with rock quality designation (RQD) in the Archean rocks. It can be observed that as RQD increases the penetration rate decreases exponentially as

$$PR = 3.7778e^{-0.014RQD}$$
(1)  
and R<sup>2</sup>=0.8101

Figure 3 shows the variation of TBM penetration rate with RQD in the Meta-sedimentary rock group. It can be observed that penetration rate varies with RQD as a polynomial function of second order with

$$PR = -0.0015 RQD^{2} + 0.1612 RQD - 1.9264$$
(2)  
and R<sup>2</sup> = 0.9732.

Second order equation gives a better degree of fit when compared to first order equation.  $R^2$  values of 0.81 and 0.97 show a good correlation between TBM penetration rate and RQD.



Fig. 2. Variation of Penetration Rate (PR) with RQD in Archean Rocks

Table 2: Rockmass Parameters of Tunnel - 1 from Inlet end

CHAINAGE (M)	RQD	$J_{N}$	RQD / JN	$J_{R}$	$J_{\scriptscriptstyle A}$	${f J_R}/{f J_A}$	$J_{\rm w}$	SRF	J <sub>w</sub> / SRF	Q- Value	Jv	Rockmass Category	Rockmass Class
42	37.5	12	3.1	1	1	1	1	1	1	3.1	23.5	IV	Poor
45	47.5	10.5	4.5	1	1	1	1	1	1	4.5	20.5	III	Fair
53	50	9	5.6	1	1	1	1	1	1	5.6	19.7	III	Fair
68	50	10.5	4.8	1	1	1	1	1	1	4.8	19.7	III	Fair
88	47.5	12	4.0	1.5	1	1.5	1	1	1	5.9	20.5	III	Fair
129	50	9	5.6	1.5	1	1.5	0.66	1	0.66	5.5	19.7	III	Fair
198	52.5	9	5.8	1.5	1	1.5	0.66	1	0.66	5.8	18.9	III	Fair
207	52.5	12	4.4	1.5	1	1.5	1	1	1	6.6	18.9	III	Fair
283	52.5	9	5.8	1	1	1	1	1	1	5.8	18.9	III	Fair
336	52.5	9	5.8	1	1	1	1	1	1	5.8	18.9	III	Fair
419	57.5	9	6.4	1	1	1	1	1	1	6.4	17.4	III	Fair
529	57.5	9	6.4	1	1	1	1	1	1	6.4	17.4	III	Fair
590	57.5	9	6.4	1	1	1	1	1	1	6.4	17.4	III	Fair
669	60	9	6.7	1	1	1	1	1	1	6.7	16.7	III	Fair
698	60	9	6.7	1	1	1	1	1	1	6.7	16.7	III	Fair
744	60	9	6.7	1	1	1	1	1	1	6.7	16.7	III	Fair
843	55	12	4.6	1.5	1	1.5	1	1	1	6.9	18.2	III	Fair
1107	62.5	12	5.2	1.5	1	1.5	1	1	1	7.8	15.9	III	Fair
1178	52.5	12	4.4	1.5	1	1.5	1	1.25	0.8	5.3	18.9	III	Fair
1259	62.5	9	6.9	1	1	1	1	1	1	6.9	15.9	III	Fair
1560	65	9	7.2	1	1	1	1	1	1	7.2	15.2	III	Fair
1845	60	15	4.0	1.5	1	1.5	1	1	1	6.0	16.7	III	Fair
2030	60	12	5.0	1	1	1	1	1	1	5.0	16.7	III	Fair
2165	50	12	4.2	1	1	1	1	1	1	4.2	19.7	III	Fair
2295	55	9	6.1	1	1	1	1	1	1	6.1	18.2	111	Fair
2365	47.5	12	3.96	1	1	1	1	1	1	3.96	20.5	IV	Poor
2511	62.5	9	6.94	1.5	1	1.5	1	2.5	0.4	4.17	15.9	III	Fair
2614	60	9	6.67	1	1	1	1	1	1	6.67	16.7	111	Fair
2740	60	6	10.00	1	1	1	1	1	1	10.00	16.7	III	Fair
2833	62.5	12	5.21	1.5	1	1.5	1	1	1	7.81	15.9	III	Fair
2944	60	9	6.67	1.5	1	1.5	0.66	1	0.66	6.60	16.7	III	Fair
3046	47.5	12	3.96	1.5	1	1.5	1	1.25	0.8	4.75	20.5		Fair
3254	57.5	6	9.6	1	1	1	1	1	1	9.6	17.4		Fair
3391	47.5	12	4.0	I	l	1	1	1.25	0.8	3.2	20.5	IV	Poor
3907	67.5	10.5	6.4	1.5	1	1.5	1	1	l	9.6	14.4		Fair
4056	60	12	5.0	1.5	1	1.5	1	1	l	7.5	16.7		Fair
4156	62.5	6	10.4	1	1	1	1	1	1	10.4	15.9	11	Good
42/1	67.5	6	11.3	1	1	1	1	1	1	11.3	14.4	11	Good
4402	57.5	9	6.4	1.5	1	1.5	1	1	1	9.6	1/.4	111	Fair
4601	60	9	6.7	1	1	1	1	1	1	0./	16./	111	Fair
4828	65	9	1.2	1.5	1	1.5	1	1	1	10.8	15.2	11	Good
4924	60	10.5	5.7	1.5	1	1.5	1	1	1	8.0	10.7	111	Fair
5020	60 57 5	10.5	5.7	1.5	1	1.5	1	1	1	8.0	10./	111	Fair
5206	57.5	9	0.4 5.2	1	1	1	1	1	1	6.4 5.2	1/.4	111	Fair
5311	50 50 5	10.5	5.2	1	1	1	1	1	1	5.2	18.2	111	Fair
5389	52.5	9	5.8	1.5	1	1.5	1	1	1	8.8 6.7	16.9	111	Fair
5700	50 5	9	0.7	1	1	1	1	1	1	0.7	10.7	111	Fair
6064	52.5	9	5.8	1	1	1	1	1	1	5.8	18.9	111	Fair
6202	52.5	10.5	5.0	1	1	1	1	1	1	5.0	16.9	111	Fair
6302	62.5	9	6.7	1.5	1	1.5	1	1	1	10.0	10.7	111	Fair
6413	62.5	9	0.9	1	1	1	1	1	1	0.9	15.9	111	Fair
040/	02.5	0	10.4	1	1	1	1	1	1	10.4	15.9	11	Good
0881	70	9	/.8	1	1	1	1	1	1	/.8	13.0	111	Fair
7004	70	10.5	0.7	1.5	1	1.5	1	1	1	10.0	12.0	11	Good
7202	72.5	9	8.1 8.6	1	1	1	1	1	1	0.1	12.9	111	Fair
7302	11.3 775	9	0.0	1.3	1	1.5	1	1	1	12.9	11.4	11	Good
7400	11.5	0	12.9	1.5	1	1.5	1	1	1	19.4	11.4	11	Cord
/408 7721	65 65	0	14.2	1.5	1	1.5	1	1	1	21.3 0.2	9.1 15.2	11	Good Esin
//31	00 50 F	10.5	0.2	1.5	1	1.5	1	1	1	9.3	13.2	111	r air
///0	52.5	12	4.4	1.5	1	1.5	1	2.5	0.4	2.0	18.9	1V	Poor
8200	02.5	0	10.4	1	1	1	1	1	1	10.4	15.9	11	G000
8200	6U	12	5.0	1.5	1	1.5	1	1	1	/.5	16.7	111	Fair
8098	05	9	/.2	1.5	1	1.5	1	1	1	10.8	15.2	11	Good
0/00	00	0	10.8	1	1	1	1	1	1	10.8	15.2	11	Good Esin
8800	00	0	10.0	1	1	1	1	2.5	0.4	4.0	10.7	111	Fair
8837	67.5	9	1.5	1	1	1	0.66	1	0.66	5.0	14.4	111	Fair

Table 3: Rockmass Parameters of Tunnel - 1 Outlet end

Reduced Distance,	Chainage (M)	RQD	J <sub>N</sub>	RQD /	J <sub>R</sub>	$\mathbf{J}_{\mathrm{A}}$	JR/	$J_w$	SRF	J <sub>w</sub> /	Q- Value	JV	Rockmass	Rockmass
RD (M)	(141)			JN			J <sub>A</sub>			SICI	value		category	01035
0	43931	80	18	4.4	2	1	1.5	1	1	1.0	6.7	10.6	III	Fair
2.5	43928.5	80	18	4.4	2	1	1.5	1	1.3	0.8	5.3	10.6		Fair
10	43921 43917	80 85	9	0.9 0.4	2	1	1.5	0.7	1	0.7	0.0 9.4	9.1		Fair
19.5	43911.5	85	12	7.1	2	1	1	1	2.5	0.4	4.3	9.1	III	Fair
28.5	43902.5	90	9	10.0	2	1	1	1	1	1.0	12.0	7.6	II	Good
136.5	43794.5	95	3	31.7	2	1	1.5	0.7	1	0.7	31.4	6.1	II	Good
524	43407	70	6	11.7	1	1	1	1	1.5	0.7	7.8	13.6	III	Fair
1125	42806	87.5	6	14.6	2	1	1.5	1	1	1.0	21.9	8.3	II II	Good
1505	42308	90	3	30.0	1	1	1	1	1	1.0	30.0	7.0	11	Good
1874	42057	95	4	23.8	2	1	15	1	1	1.0	35.6	6.1	II	Good
2000	41931	87.5	4	21.9	2	1	1.5	1	1	1.0	32.8	8.3	II	Good
2092.5	41838.5	85	4	21.3	2	1	1.5	1	1	1.0	31.9	9.1	II	Good
2168.9	41762.1	77.5	4	19.4	2	1	1.5	1	1	1.0	29.1	11.4	II	Good
2262	41669	87.5	4	21.9	2	1	1.5	1	1	1.0	32.8	8.3	II	Good
2740	41191	85 87 5	6	14.2	1	1	1	1	1	1.0	14.2	9.1		Good
2820	41021	87.5 77 5	9	29.2 8.6	2	1	15	1	1	1.0	12.9	0.5 11 4	II	Good
3021.9	40909.1	82.5	4	20.6	2	1	1.5	1	1	1.0	30.9	9.8	II	Good
3144.2	40786.8	77.5	4	19.4	1	1	1	1	1	1.0	19.4	11.4	II	Good
3324	40607	72.5	9	8.1	2	1	1.5	1	1	1.0	12.1	12.9	II	Good
3568	40363	87.5	4	21.9	2	1	1.5	1	1	1.0	32.8	8.3	II	Good
3682	40249	77.5	6	12.9	2	1	1.5	1	1	1.0	19.4	11.4	II II	Good
3835	40096	82.5 80	4	20.6	2	1	1.5	1	1	1.0	30.9	9.8		Good
3941 5	39989 5	77 5	9	20.0	2	1	1.5	1	1	1.0	12.9	11.4	II	Good
4098.6	39832.4	72.5	11	6.9	2	1	1.5	1	1	1.0	10.4	12.9	II	Good
4410.6	39520.4	50	15	3.3	2	1	1.5	0.7	1	0.7	3.3	19.7	IV	Poor
4539.3	39391.7	57.5	11	5.5	2	1	1.5	1	1	1.0	8.2	17.4	III	Fair
4575.6	39355.4	77.5	11	7.4	2	1	1.5	1	1	1.0	11.1	11.4	II	Good
4624.4	39306.6	75 72 5	6	12.5	2	1	1.5	1	1	1.0	18.8	12.1	II II	Good
4/40.2	39190.8	72.5 67.5	4	18.1	2	1	1.5	1	1	1.0	16.9	12.9	11 11	Good
5103.5	38827.5	65	6	10.8	2	1	1.5	1	1	1.0	16.3	15.2	II	Good
5140.3	38790.7	65	6	10.8	2	1	1.5	1	1	1.0	16.3	15.2	II	Good
5230	38701	67.5	4	16.9	2	1	1.5	1	1	1.0	25.3	14.4	II	Good
5293.5	38637.5	70	4	17.5	2	1	1.5	1	1	1.0	26.3	13.6	II	Good
5314.8	38616.2	75	4	18.8	2	1	1.5	1	1	1.0	28.1	12.1	II	Good
53/2.5	38558.5	60 72 5	6	10.0	2	1	1.5	1	1	1.0	15.0	16.7		Good
5455.6	38475.4	80	6	12.1	2	1	1.5	1	1	1.0	20.0	12.9	II	Good
5487.3	38443.7	80	6	13.3	2	1	1.5	1	1	1.0	20.0	10.6	II	Good
5566	38365	65	6	10.8	2	1	1.5	1	1	1.0	16.3	15.2	II	Good
5666	38265	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
5711.8	38219.2	77.5	6	12.9	2	1	1.5	1	1	1.0	19.4	11.4	II	Good
5736.4	38194.6	75	6	12.5	2	1	1.5	1	1	1.0	18.8	12.1		Good
5811	38120	73 77 5	6	12.5	2	1	1.5	1	1	1.0	10.0	12.1	11 11	Good
5871.7	38059.3	80	4	20.0	2	1	1.5	1	1	1.0	30.0	10.6	II	Good
5924.6	38006.4	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
5977.8	37953.2	72.5	6	12.1	2	1	1.5	1	1	1.0	18.1	12.9	II	Good
6031.6	37899.4	57.5	12	4.8	2	1	1.5	1	1	1.0	7.2	17.4	III	Fair
6072.6	37858.4	62.5	9	6.9	2	1	1.5	1	1	1.0	10.4	15.9	II	Good
6149.65	37781 35	57.5 60	9	6.4 6.7	2	1	1.5	1	1	1.0	9.0	167	П	Good
6189	37742	60	9	67	2	1	1.5	1	1	1.0	10.0	16.7	II	Good
6214	37717	50	12	4.2	2	1	1.5	1	1.3	0.8	5.0	19.7	III	Fair
6227	37704	60	9	6.7	2	1	1.5	1	1	1.0	10.0	16.7	II	Good
6383.1	37547.9	65	9	7.2	2	1	1.5	1	1	1.0	10.8	15.2	II	Good
6430	37501	50	12	4.2	2	1	1.5	1	1.3	0.8	5.0	19.7	III	Fair
6609.6	3/321.4	67.5	6	7.5	2	1	1.5	1	1	1.0	10.9	14.4		Good
6873 3	37057 7	60	9	10.0	2	1	1.5	1	1	1.0	15.0	14.4	П	Good
7024	36907	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
7146	36785	62.5	12	5.2	2	1	1.5	1	1	1.0	7.8	15.9	III	Fair
7220.2	36710.8	57.5	12	4.8	2	1	1.5	1	1	1.0	7.2	17.4	III	Fair
7287	36644	47.5	12	4.0	2	1	1.5	0.7	1	0.7	3.9	20.5	IV	Poor
7293	36638	40	12	3.3	2	1	1.5	0.7	1	0.7	3.3	22.7	IV	Poor
1320 7332.06	30011 36598.04	40 47 5	15	2./ 4.0	2	1	1.5	1	1.5	0.8	5.2 5.9	22.7	1 V	roor Fair
7397	36534	45	11	4.3	2	1	1.5	1	1	1.0	6.4	20.3	III	Fair
	-	-										.=	-	Contd

Reduced Distance, RD (M)	Chainage (M)	RQD	J <sub>N</sub>	RQD /	J <sub>R</sub>	J <sub>A</sub>	$JR/J_{A}$	$\mathbf{J}_{\mathrm{w}}$	SRF	J <sub>w</sub> / SRF	Q- Value	Jv	Rockmass category	Rockmass Class
7456	36475	55	11	5.2	2	1	1.5	1	1	1.0	7.9	18.2	III	Fair
7481	36450	57.5	9	6.4	2	1	1.5	1	1	1.0	9.6	17.4	III	Fair
7505.7	36425.3	52.5	9	5.8	2	1	1.5	1	1	1.0	8.8	18.9	III	Fair
7524.8	36406.2	60	9	6.7	2	1	1.5	1	1	1.0	10.0	16.7	II	Good
7635	36296	65	9	7.2	2	1	1.5	1	1	1.0	10.8	15.2	II	Good
7723	36208	57.5	9	6.4	2	1	1.5	1	1	1.0	9.6	17.4	III	Fair
7747.3	36183.7	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
7832	36099	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
7852.4	36078.6	70	6	11.7	2	1	1.5	1	1	1.0	17.5	13.6	II	Good
7922	36009	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
7978	35953	70	6	11.7	2	1	1.5	1	1	1.0	17.5	13.6	II	Good
8056	35875	75	4	18.8	2	1	1.5	1	1	1.0	28.1	12.1	II	Good
8094.6	35836.4	75	4	18.8	2	1	1.5	1	1	1.0	28.1	12.1	II	Good
8107.2	35823.8	70	6	11.7	2	1	1.5	1	1	1.0	17.5	13.6	II	Good
8190	35741	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
8235.3	35695.7	65	6	10.8	2	1	1.5	1	1	1.0	16.3	15.2	II	Good
8275	35656	65	6	10.8	2	1	1.5	1	1	1.0	16.3	15.2	II	Good
8289.85	35641.15	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	II	Good
8317	35614	77.5	4	19.4	2	1	1.5	1	1	1.0	29.1	11.4	II	Good
8363	35568	77.5	6	12.9	2	1	1.5	1	1	1.0	19.4	11.4	II	Good
8440.2	35490.8	67.5	6	11.3	2	1	1.5	1	1	1.0	16.9	14.4	11	Good
8502	35429	62.5	12	5.2	2	1	1.5	1	1	1.0	7.8	15.9	111	Fair
86/6	35255	80	3	26.7	2	1	1.5	1	1	1.0	40.0	10.6	1	Very Good
8/18.2	35212.8	72.5	0	12.1	2	1	1.5	1	1	1.0	18.1	12.9	11	Good
0/01	25112	70	11	67	2	1	1.5	1	1	1.0	17.5	13.0	11	Good
0010 9961 9	35060.2	70	6	0.7	2	1	1.5	1	1	1.0	10.0	13.0	111	Fall
8015	35016	70	6	11.7	2	1	1.5	1	1	1.0	17.5	13.0	11	Good
8942 3	34988 7	72 5	6	12.1	1	1	1.5	1	1	1.0	12.1	12.0	11	Good
8981.2	34949.8	72.5	6	12.1	2	1	15	1	1	1.0	18.1	12.9	II	Good
9022.79	34908 21	67.5	12	5.6	2	1	1.5	1	1	1.0	8.4	14.4	III	Fair
9058.13	34872.87	65	9	7.2	2	1	1.5	1	1.3	0.8	8.7	15.2	III	Fair
9087.3	34843.7	47.5	9	5.3	2	1	1.5	1	1.3	0.8	6.3	20.5	III	Fair
9122	34809	52.5	12	4.4	2	1	1.5	1	1	1.0	6.6	18.9	III	Fair
9154.2	34776.8	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
9200	34731	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
9224.6	34706.4	72.5	6	12.1	2	1	1.5	1	1	1.0	18.1	12.9	II	Good
9268.5	34662.5	72.5	6	12.1	2	1	1.5	1	1	1.0	18.1	12.9	II	Good
9287	34644	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
9298	34633	72.5	6	12.1	2	1	1.5	1	1	1.0	18.1	12.9	II	Good
9367	34564	67.5	9	7.5	2	1	1.5	1	1	1.0	11.3	14.4	II	Good
9421.5	34509.5	80	6	13.3	2	1	1.5	1	1	1.0	20.0	10.6	II	Good
9473.9	34457.1	70	9	7.8	2	1	1.5	1	1	1.0	11.7	13.6	II	Good
9523.3	34407.7	62.5	9	6.9	2	1	1.5	1	1	1.0	10.4	15.9	II	Good
9570.15	34360.85	52.5	12	4.4	2	1	1.5	1	1	1.0	6.6	18.9	III	Fair
9653	34278	62.5	9	6.9	2	1	1.5	1	1	1.0	10.4	15.9	II	Good
9682.1	34248.9	62.5	9	6.9	2	1	1.5	1	1	1.0	10.4	15.9	II	Good
9724	34207	62.5	9	6.9	2	1	1.5	1	1	1.0	10.4	15.9	II	Good
9875	34056	57.5	9	6.4	2	1	1.5	1	1	1.0	9.6	17.4	111	Fair
9942.4	33988.6	70	6	11.7	2	1	1.5	1	1	1.0	17.5	13.6	11	Good
9984.5	33946.5	90	4	22.5	2	1	1.5	1	1	1.0	33.8	7.6	11	Good
10009	33922	75	6	12.5	2	1	1.5	1	1	1.0	18.8	12.1	11	Good
10036.6	33894.4 22004 54	12.3	6	12.1	2	1	1.5	1	1	1.0	18.1	12.9	11	Good
10040.40	55884.54	15	0	12.5	2	1	1.5	1	1	1.0	18.8	12.1	11	0000

## Variation of TBM Penetration Rate with Unconfined Compressive Strength (UCS)

Unconfined Compressive strength (UCS) is one of the most common parameters of describing the intact rock strength.TBM penetration is possible only when the applied forces exceed the UCS of the rockmass (Roxborough and Phillips, 1975). UCS is also one of the factors influencing rockabrasivity (Plinninger, 2010).

Figure 4 shows the variation of penetration rate with unconfined compressive strength (UCS) in the Archaean formation from the Outlet end performed on intact rock samples. As stressstrain behaviour of rock is nonlinear and because of medium strong (R3) to very strong (R5) rock (ISRM1981) encountered in tunnelling there is considerable scatter of data. A linear fit for PR with UCS gives very low  $R^2$  Accordingly, to obtain a better fit, a polynomial function of order three is given in equation (3).

 $PR = -3E-11UCS^3 + 6E-08UCS^2 + 0.0001UCS + 1.3326$  (3) with  $R^2$  of 0.0341.-

Figure 5 shows the variation of penetration rate with UCS in the metasedimentary formation from the Inlet end. The penetration rate decreases with UCS as a polynomial function of order two given by

$$PR = -1E - 05UCS^{2} + 0.0091UCS + 0.9413$$
(4)  
with a R<sup>2</sup> value of 0.69 showing good correlation.



Fig. 3. Variation of Penetration Rate (PR) with RQD in Meta-sedimentary Rocks

It can be inferred that TBM penetration rate shows a good correlation with UCS in Meta Sedimentary rock group and poor correlation in the medium to high strength crystalline rocks of the Archean group. The reason for poor correlation in the granites/gneisses may be due to the wear of cutter discs when boring in rockmass of variable grades fromR3 to R5 as compared to the metasedimentary rocks where only medium strong (R3) to strong rock (R4) was encountered.

# Variation of Porosity with Unconfined Compressive Strength (UCS)

Mikhail (2020) studied the effect of porosity on the unconfined compressive strength of Kuznetsk Basin sandstones in Siberia, Russia. Figure 6 shows the variation of porosity with unconfined compressive strength (UCS) in the metasedimentary. It can be seen that UCS decreases as a second-order polynomial function with an increase in porosity in the metasedimentary rocks, as shown in Figure 6 and given by equation (5) with  $R^2 = 0.5765$ .

$$UCS=193.4POROSITY^{-0.656}$$
(5)

### Variation of TBM Penetration Rate with Volumetric Joint Count (Jv) and Tensile Strength

During the TBM boring process, presence of joints will have a favourable effect on the penetration rate up to a certain spacing (Bruland, 1998; Wilfing, 2016).

Figure 7a-b shows the variation of penetration rate with volumetric joint count (Jv) in the Archeans and Meta-sedimentary. Jv is obtained from RQD from the formula RQD =115-3.3Jv when



Fig. 5. Variation of Penetration Rate with Unconfined Compressive Strength (UCS) in Meta-sedimentary Rocks



Fig. 4. Variation of Penetration Rate with Unconfined Compressive Strength (UCS) in Archean Rocks

rock cores are not available (IS13365, Part1:1998). It can be observed that penetration rate increases exponentially with (Jv) in the archean sand meta-sedimentary rock groups with  $R^2 = 0.8167$  and 0.8275, respectively and shown by equations (6) and (7).

$$PR = 0.7047e^{0.0491Jv}$$
(6)

$$PR = 0.7416e^{0.0634Jv}$$
(7)

Figure 8a-b show the variation of Jv with tensile strength determined in the laboratory for 17 core samples on intact rock. It can be observed that in Archean Group, Jv varies as a polynomial function of the tensile strength of the rock with  $R^2$  of 0.0562 showing poor correlation.

$$\text{Fensile Strength} = -0.1954 \text{Jv}^2 + 5.1371 \text{Jv} + 57.234 \qquad (8)$$

Figure 8b shows the variation of Jv with tensile strength in the meta-sedimentary group. The Jv shows a better correlation with Tensile strength in the metasedimentary when compared to Archean, and it varies as a second order polynomial with  $R^2$  of 0.2349

Tensile strength = 
$$-7.2963 \text{Jv}^2 + 266.47 \text{Jv} - 2342$$
 (9)

### Variation of TBM Penetration Rate with Rockmass Class

Rockmass characterisation with Q-system(IS13365, Part2: 1992) provides a field method of classifying the rockmass into five classes from Class I (Very Good Rock) to V(Very Poor rock). RQD/Jn is a crude measure of the block size where Jn is the Joint set number obtained from face logging of excavation.

Figure 9 shows the variation of rockmass class with TBM penetration rate in the Archean Group. It is evident that higher



Fig. 6. Variation of Porosity with Unconfined Compressive Strength (UCS) in Meta-sedimentary Rocks



Fig. 7. Variation of Penetration Rate with (a) J<sub>v</sub> in Archean Rocks (b) J<sub>v</sub> in Meta-sedimentary Rocks



Fig. 8. Variation of Tensile Strength with (a) J<sub>v</sub> in Archeans Rocks (b) J<sub>v</sub> in Meta-sedimentary Rocks

penetration rates are achieved in Class III and ClassIV rock when compared with Class I and II. Figure 10 shows the variation of rockmass class with penetration rate in the meta-sedimentary formation. It can be observed that higher penetration rate was obtained in Class III rock when compared to Class II rock.

### Conclusions

TBM penetration rate in hard rocks comprising of granites, granite gneisses, quartzites, sandstones and quartz arenites of Archean and meta-sedimentary rock groups was investigated. It is inferred that penetration rate shows a good correlation with RQD for both Archean and meta-sedimentary rock groups as well as with UCS in the Meta Sedimentary rock group but poorly in the medium to very strong crystalline rocks of the Archean Group. Penetration rate increases exponentially with joint volume in both the rock groups. Volumetric joint count (Jv) does not show any significant correlation with Tensile strength in both the rock groups with relatively better correlation in the Meta-sedimentary group. Porosity increases exponentially with decrease in UCS in the metasedimentary group.



Fig. 9. PR with Rock Mass Class in Archean Rocks

### **Authors' Contributions**

**RD**: Investigation, Conceptualisation, Methodology, Writing-Original Draft, Formal Analysis. **RRN**: Supervision, Reviewing and Editing.

### **Conflict of Interest**

The authors declare that they have no conflicts of interest.

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Fig. 10. PR with Rock Mass Class in Meta-sedimentary Rocks

#### References

- Armaghani, D.J., Yagiz, S., Mohamad, E.T. and Zhou, J. (2021). Prediction of TBM performance in fresh through weathered granite using empirical and statistical approaches. Tunnel. Undergr. Space Technol., v. 118, 10483, pp.1-26.
- Barton, N. (1999). TBM performance estimation in rock using  $Q_{\text{TBM}}$ . Tunnels and Tunnelling International. pp. 30-34
- Barton, N., Lien, R. and Lunde, J. (1974). Engineering Classification of rockmasses for the design of tunnel support. Jour. Rock Mech., v. 64, pp.189-236.
- Blindheim, O.T. (1979). Boreability Predictions for Tunneling. Ph.D. Thesis, Department of Geological Engineering, The Norwegian Institute of Technology, 406p.
- Bieniawski, Z.T., Celada, B. and Galera, J.M. (2007). Predicting TBM excavability. Tunnels and Tunneling International, pp. 25-28.
- Bruland, A. (1998). Hard Rock Tunnel Boring. Ph.D.Thesis, Norwegian University of Science and Technology, Trondheim, Norway.
- Das, Sankha and Chakraborty, Munmun (2019). Petrology and geochemistry of mafic dyke and sills in Cumbum Formation of the Proterozoic Nallamalai fold belt, Rajampet area, Andhra Pradesh, India. Jour.Earth Syst. Sci. v.128: 116, pp.1-22, https://doi.org/ 10.1007/s12040-019-1102-0
- Gong, Q.M. and Zhao, J. (2009). Development of a rockmass characteristics model for TBM penetration rate prediction. Internatl. Jour. Rock Mech. Min. Sci., v. 46(1), pp. 8-18.
- Hamidi, J., Kourosh, S., Bahram, R. and Jamal, R. (2010). Performance prediction of hard rock TBM using rockmass Rating (RMR) system. Tunnel. Undergr. Space Technol., v. 25, pp. 333-345.
- Hossein, A., Hashemi, M. and Ajalloeian, R. (2020). Evaluation of geotechnical parameters affecting the penetration rate of TBM using neural network (case study). Arab. Jour. Geosci., v.13 (183), pp. 1-11
- IS11315 (Parts 1-11) (1987; Reaffirmed in 2001). Indian Standard Quantitative Description of Discontinuities in rockmasses. Bureau of Indian Standards.
- IS13365 (Part 2) (1992; Reaffirmed in 2001). Indian Standard Quantitative Classification systems of rockmass-Guidelines, Part 2, rockmass Quality for Prediction of Support Pressure in Underground Openings. Bureau of Indian Standards, pp.1-8.
- Jain, Prasanna (2014). Evaluation Of Engineering Geological and Geotechnical Properties for The Performance of a Tunnel Boring Machine in Deccan Trap Rocks – A Case Study from Mumbai, India. Ph.D. Thesis, Indian Institute of Technology, Mumbai.
- Khetwal, A., Rostami, J., Frough, O. and Nielsen, P.P. (2021).Comparison between discrete event simulation approach and various existing empirically based models for estimation of TBM utilization. Tunnel. Undergr. Space Technol., v.112,103883., pp.1-13.
- King, W. (1872) Kudapah and Karnul Formations in the Madras Presidency. Mem. Geol. Surv. India, v.8, pp.1-346.
- Lislerud, A.(1988).Hard rock tunnel boring: prognosis and costs. Tunnel. Undergr. Space Technol., v.3(1), pp.9-17.
- Mandapalli, Raju (1989). A geotechnical note on the Srisailam left bank canal scheme, Nalgonda District, Andhra Pradesh. Rec. Geol. Surv. India, v.122(5), pp. 261-262.
- Mandapalii, Raju (2008). A geotechnical note on Srisailam Left Bank Canal Tunnel of Alimineti Madhava Reddy Project, Andhra Pradesh. Rec.Geol. Surv. India, v.140(5), pp.143-145.
- Mandapalli, Raju (2009). Geotechnical note on Srisailam Left Bank Canal Tunnel alignment of Alimineti Madhava Reddy Project, Andhra Pradesh. Rec.Geol. Surv. India, v.141(5), pp.166-167.
- Meijerink, A.M.J., Rao, D.P. and Rupke, J. (1984). Stratigraphic and structural development of the Precambrian Cuddapah basin, SE India. *Precamb. Res.*, v.26, pp.57-104.

- Mikhail, Eremin (2020). Influence of the porosity on the uniaxial compressive strength of sandstone samples. Proced. Struct. Integrit., v. 25, pp. 465-469.
- Nagaraja, Rao, B.K., Rajurkar, S.T., Ramalingaswamy, G. and Ravindra Babu, B. (1987). Stratigraphy and structure and evolution of the Cuddapah Basin. *In*: Purana Basins of Peninsular India (Middle to Late Proterozoic). Jour. Geol. Soc. India., v.6, pp. 33-86.
- Patranabis-Deb, S., Saha, D, and Tripathy, V. (2012). Basin stratigraphy, sea level fluctuations and their global tectonic connections - Evidence from the Proterozoic Cuddapah Basin. *Geol. Jour.*, Wiley Online Library, DOI: 10.1002/gj.1347
- Pells, P.J., Bieniawski, Z.T., Hencher, S.R. and Pells, S.E. (2017). Rock quality designation (RQD): Time to rest in peace. Canadian Geotech. Jour., v. 54(6), pp. 825–834. https://doi.org/10.1139/cgj-2016-0012
- Plinninger, R.J. (2010). Hard rock abrasivity investigation using the rock abrasivity index (RAI). Geologically Active-Williams et al. (*Eds.*), Taylor and Francis Group, London, ISBN 978-0-415-60034-7, pp.3445-3452
- Qiuming, Gong, Hongi, Xu, Jianwei, Lu, Fan, Wu, Xiaoxiong, Zhou and Lijun, Yin (2022). Rockmass characteristics model for TBM penetration rate prediction-an updated version. Internatl. Jour. Rock Mech. Min. Sci., v.149,104993, pp.1-11.
- Raju, K.C.C. (2009).Geotechnical Evaluation of Design Parameters for Use in the Tunnel Boring Machines of Srisailam Left Bank Tunnel with the help of Satellite Imageries, Mahbubnagar District, Andhra Pradesh. Jour. Engineer. Geol., v. 36(1-4), pp.207-216.
- Rajurkar, S.T., and Ramalingaswami, G. (1975). Facies variation within the upper Cuddapah strata in the northern part of Cuddapah Basin. In: Precambrian Geology of the Peninsular Shield, Part 1. Miscellaneous Publication, Geol. Surv. India, v. 23, pp.157-164.
- Ramakrishna, D. and Rao, N.R. (2023). Geotechnical Investigation and Monitoring of Underground Excavation of Tunnel-2 in Srisailam Left Bank Canal Tunnel Project (AMRP), Nalgonda District, Telangana, India. Jour. Geosci. Res., v.8(1), pp.18-24. https://doi.org/ 10.56153/g19088-022-0092-19.
- Ramakrishnan, M. and Vaidyanathan, R. (2008). Geology of India, Volume 1, Geological Society of India, Bangalore.
- Rostami, J. and Ozdemir, L. (1993b). A New Model for Performance Prediction of Hard Rock TBM. *In:* Proceedings of Rapid Excavation and Tunnelling Conference. Chapter 50, Boston, MA. USA, 793-809.
- Rostami, J. and Ozdemir, L. (1993c). Computer Modeling for Cutterhead Design and Layout of Mechanical Excavators. Proceedings of Annual Technical Meeting of the Institute of Shaft Drilling Technology.Las Vegas, NV, USA.
- Rostami, J. (1997). Development of a Force Estimation Model for Rock Fragmentation with Disc Cutters Through Theoretical Modeling and Physical Measurement of Crushed Zone Pressure. Ph. D. Thesis, Colorado School of Mines, Golden, Colorado, USA, 249p.
- Roxborough, F. and Phillips, H. (1975): Rock excavation by disc cutter. Int. J. Rock Mech. Min. Sci. and Geomech. Abstr., v.12(12), pp.361–366.
- Sen, S.N. and Narsimha Rao, Ch. (1967). Igneous activity in Cuddapah Basin and adjacent area and suggestions on the paleo-geography of the basin. Proceedings of Symposium: Upper Mantle Project 8, GRB and NGRI publication, Hyderabad, pp.261-285.
- Wilfing, L. (2016). The Influence of Geotechnical Parameters on Penetration Prediction in TBM Tunneling in Hard Rock., Dr.-Ing. Thesis., Technische Universität München, 191p.
- Yagiz, S. (2008).Utilising rockmass properties for predicting TBM performance in hard rock condition. Tunnelling and underground space technology, 23(2008), pp.326-339.